

DRAFT

**VALLEY GENERATING STATION
TRAFFIC IMPACT ANALYSIS**

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Chapter 1.0 INTRODUCTION

1.1 INTRODUCTION

This report presents the results of a traffic analysis performed for the proposed modifications to a Los Angeles Department of Water and Power (LADWP) power generating station. This report has been prepared for submittal in support of the Environmental Impact Report for the proposed project site.

1.2 PROJECT DESCRIPTION

The LADWP is proposing modifications to the Valley Generating Station (VGS) to help comply with its annual Regional Clean Air Incentives Market (RECLAIM) Allocations for future years, improve in-Basin power reliability, and participate in the Californian Independent System Operator (“Cal-ISO”) by supplying excess electrical power, thereby reducing the risk of blackouts for the state.

1.2.1 Project Location

The VGS is located at 9430 San Fernando Road in the City of Los Angeles (Sun Valley) (Figure 1-1). The VGS occupies a parcel of land bounded by Glenoaks Boulevard to the northeast; Sheldon Street to the southeast; San Fernando Road to the southwest; and a flood control channel to the northwest, beyond which is Branford Road. The area surrounding the facility is primarily commercial/industrial; however, an emergency medical clinic, a hospital and two motels are present adjacent to the site on San Fernando Road. A sand and gravel plant is located adjacent to the northwest of the site. There are no residences in the immediate vicinity of the facility, with the nearest residential properties located approximately one-half mile north of the site.

Figure 1-1

1.3 ANALYSIS SCOPE

The traffic analysis examines the impacts of adding construction project generated traffic to existing traffic on the surrounding arterial network. The proposed project is subject to the requirements of the South Coast Air Quality Management Department (SCAQMD), the Los Angeles Department of Transportation (LADOT) and the Los Angeles County Congestion Management Program (LACOCMP).

The SCAQMD guidelines for project impacts are based on the following significance criteria:

- For project impacts that would last between three and 12 months, V/C ratio increase greater than or equal to 0.04, if LOS is E or worse.
- For project with impacts longer than 12 months if V/C ratio increase greater or equal to 0.040 and LOS is C: V/C ration increase greater than or equal to 0.020 and LOS is D; and V/C ratio increase greater than or equal to 0.010 and LOS is E or F.
- A major roadway or railroad is closed to all through traffic and no alternate route is available.
- The project will increase customer traffic to a facility by more than 700 trips per day.

Nineteen critical study area intersections along the roadways forecast to be used and possibly impacted by project traffic have been identified for analysis and include:

1. Glenoaks Blvd & Sheldon St	11. SR-170 SB ramp & Arleta Ave
2. Glenoaks Blvd & Tuxford St	12. I-5 NB ramp & Sunland Blvd
3. Sunland Blvd & Glenoaks Blvd	13. I-5 SB ramp & Sunland Blvd
4. San Fernando & Osborne St	14. I-5 NB ramp & Sheldon St
5. San Fernando & Sheldon St	15. I-5 SB on/off ramp & Laurel Cyn
6. Arleta Ave & Sheldon St	16. I-5 NB ramp & Laurel Cyn
7. Coldwater Cyn & Roscoe	17. I-5 NB on/off ramp & Osborne St
8. SR-170 NB ramp & Roscoe	18. I-5 SB on/off ramp & Osborne St
9. SR-170 SB ramp & Roscoe	19. Laurel Cyn & Sheldon St
10. SR-170 NB off & Sheldon St	

Table 1-1 summarizes the definitions of the various levels of service. The LOS for all intersections are calculated using the intersection capacity utilization (ICU) methodology using the following capacities:

Capacity: 1600 vehicles per hour per lane for through and single-turn lanes

2880 vehicles (total) per hour for dual turn lanes.

Clearance interval: 0.10

Source: 1999 LACO CMP Guideline Criteria

1.4 DEFINITIONS

Certain terms used throughout this report are defined below to clarify their intended meaning:

- ADT - Average Daily Traffic.
- ICU - Intersection Capacity Utilization. A factor used to measure the volume to capacity ratio for an intersection and determine the level of service.
- LOS - Level of Service. A scale used to evaluate circulation system performance based on intersection ICU values or volume/capacity ratios of arterial segments. The levels range from "A" to "F", with LOS "A" representing free flow traffic and LOS "F" representing severe traffic congestion.
- Peak Hour - This typically refers to the hour during the morning AM peak period (typically 7 AM - 9 AM) or the afternoon PM peak period (typically 3 PM - 6 PM) in which the greatest number of vehicle trips are generated by a given land use or are travelling on a given roadway.
- VPD - Vehicles per Day. This has the same meaning as ADT but is generally used in a trip generation context rather than in reference to the highway volume of an arterial segment.
- VPH - Vehicles per Hour.
- V/C - Volume to Capacity Ratio. This is typically described as a percentage of capacity utilized by existing or projected traffic on a segment of arterial or an intersection turn movement.

1.5 REFERENCES

1. "Trip Generation: An Informal Report" (6th Edition), Institute of Transportation Engineers, 1997.
2. "1999 Congestion Management Program for Los Angeles County," Los Angeles County Metropolitan Transportation Authority, November, 1999.

Table 1-1
LEVELS OF SERVICE FOR URBAN AND SUBURBAN LOCATIONS

LEVEL OF SERVICE (V/C)	DESCRIPTION
A 0.00-0.60	At level of service A there are no cycles which are fully loaded, and few are even close to loaded. No approach phase is fully utilized by traffic and no vehicle waits longer than one red indication. Typically the approach appears quite open, turning movements are easily made, and nearly all drivers find freedom of operation.
B > 0.60-0.70	Level of service B represents a stable operation. An occasional approach phase is fully utilized and a substantial number are approaching full use. Many drivers begin to feel somewhat restricted within platoons of vehicles.
C > 0.70-0.80	In level of service C stable operation continues. Full signal cycle loading is still intermittent, but more frequent. Occasionally drivers may have to wait through more than one red signal indication, and back-ups may develop behind turning vehicles.
D > 0.80-0.90	Level of service D encompasses a zone of increasing restriction approaching instability. Delays to approaching vehicles may be substantial during short peaks within the peak period, but enough cycles with lower demand occur to permit periodic clearance of developing queues, thus preventing excessive back-ups.
E > 0.90-1.00	Level of service E represents the most vehicles that any particular intersection approach can accommodate. At capacity (V/C = 1.00) there may be long queues of vehicles waiting upstream of the intersection and delays may be great (up to several cycles).
F > 1.00	Level of service F represents jammed conditions. Back-ups from locations downstream or on the cross street may restrict or prevent movement of vehicles out of the approach under consideration; hence, volumes carried are not predictable. V/C values are highly variable, because full utilization of the approach may be prevented by outside conditions
Source“1999 Congestion Management Program for Los Angeles County,” Los Angeles County Metropolitan Transportation Authority, November 1999.	

Chapter 2.0

PROJECT SETTING

This chapter describes the project site in relation to the transportation setting. The existing circulation system is discussed, and existing traffic volumes and levels of service are summarized.

2.1 SURROUNDING HIGHWAY NETWORK

The proposed project may affect eight primary roadways in the vicinity of the VGS through the addition of project traffic to these roadways. Following is a description of these roadways and other local circulation routes.

Golden State Freeway (Interstate 5) - is an eight-to-ten lane freeway traveling south-north from Santa Ana to Sacramento. It passes west of the site and provides interchanges at Sheldon Street, Tuxford Street, Osborne Street, and Sunland Boulevard.

Sheldon St/Coldwater Cyn - is a four-lane roadway providing direct access to the project site and extends west and south from the project site. There are a few signalized cross streets, consisting of Laurel Canyon, San Fernando Road, and Glenoaks Boulevard. All other cross street traffic is controlled by STOP signs and Sheldon Street traffic does not stop at these locations. Sheldon Street is bounded by primarily commercial/industrial uses.

Osborne Street, Tuxford Street, Sunland Boulevard - are east-west arterial roadways located west of the project site that provide access to San Fernando Road and Glenoaks Boulevard. Additionally, these roadways provide access to ramp connections with the I-5 freeway.

San Fernando Road, Glenoaks Boulevard - are north-south roadways that intersect with Osborne Street, Tuxford Street and Sunland Boulevard, thereby providing access to the project site via Sheldon Street.

Roscoe Boulevard - Roscoe Boulevard changes into Tuxford Street as it turns northeast from an east-west direction. Its ramp connections with the SR-170 provide access to the local street system via Coldwater Canyon/Sheldon Street and Roscoe Boulevard/Tuxford Street.

Construction traffic generated by the proposed project at the VGS location will access the site via Sheldon Street and will be directed along Osborne Street, Tuxford Street and Sunland Boulevard toward the I-5 freeway and Sheldon Street/Coldwater Canyon toward the SR-170 freeway.

2.2 EXISTING TRAFFIC CONDITIONS

The VGS is located at 9430 San Fernando Road in the City of Los Angeles (Sun Valley). The VGS occupies a parcel of land bounded by Glenoaks Boulevard to the northeast; Sheldon Street to the southeast; San Fernando Road to the southwest; and a flood control channel to the northwest, beyond which is Branford Road. The area surrounding the facility is primarily commercial/industrial; however, an emergency medical clinic, a hospital and two motels are present near the site on San Fernando Road. A sand and gravel plant is located adjacent to the northwest of the site. There are no residences in the immediate vicinity of the VGS with the nearest residential properties located approximately one-half mile north of the site. The following 19 intersections have been included in the traffic analysis:

- | | |
|---------------------------------|-------------------------------------|
| 1. Glenoaks Blvd & Sheldon St | 11. SR-170 SB ramp & Arleta Ave |
| 2. Glenoaks Blvd & Tuxford St | 12. I-5 NB ramp & Sunland Blvd |
| 3. Sunland Blvd & Glenoaks Blvd | 13. I-5 SB ramp & Sunland Blvd |
| 4. San Fernando & Osborne St | 14. I-5 NB ramp & Sheldon St |
| 5. San Fernando & Sheldon St | 15. I-5 SB on/off ramp & Laurel Cyn |
| 6. Arleta Ave & Sheldon St | 16. I-5 NB ramp & Laurel Cyn |
| 7. Coldwater Cyn & Roscoe | 17. I-5 NB on/off ramp & Osborne St |
| 8. SR-170 NB ramp & Roscoe | 18. I-5 SB on/off ramp & Osborne St |
| 9. SR-170 SB ramp & Roscoe | 19. Laurel Cyn & Sheldon St |
| 10. SR-170 NB off & Sheldon St | |

Existing AM and PM peak hour turning movement volumes at these intersections and existing Average Daily Traffic (ADT) volumes on selected roadway segments were counted by Traffic Data Services, Inc. and are illustrated in Figures 2-1 and 2-2. Intersection capacity utilization (ICU) values are presented in Table 2-1 (actual ICU calculations are included in Appendix A of this document) and are a means of representing peak hour volume/capacity ratios. The ICU is the proportion of an hour required to provide sufficient capacity to accommodate all intersection traffic if all approaches operate at capacity. If an intersection is operating at 80 percent of capacity, then 20 percent of the signal cycle is not used. The signal could show red on all indications 20 percent of the time and the signal would just accommodate approaching traffic.

Figure 2-1

Figure 2-2

Table 2-1

ICU SUMMARY – EXISTING CONDITIONS

INTERSECTION	AM	PM
1. Glenoaks & Sheldon	0.617	0.463
2. Glenoaks & Tuxford	0.670	0.646
3. Sunland & Glenoaks	0.834	0.697
4. San Fernando & Osborne	0.748	0.799
5. San Fernando & Sheldon	0.629	0.800
6. Arleta & Sheldon	0.490	0.754
7. Coldwater Canyon & Roscoe	0.999	1.106
8. SR-170 SB Ramp & Roscoe	0.655	1.206
9. SR-170 NB Ramp & Roscoe	0.842	0.888
10. SR-170 NB off & Sheldon	0.519	0.509
11. SR-170 SB ramp & Arleta	0.603	0.749
12. Sunland & I-5 NB on/off	0.763	0.594
13. Sunland & I-5 SB on/off	0.769	0.848
14. I-5 NB on/Rincon & Sheldon	0.637	0.575
15. Laurel Canyon & I-5 SB on/off	0.520	0.656
16. Laurel Canyon & I-5 NB off	0.449	0.500
17. I-5 NB on/off & Osborne	0.607	0.704
18. I-5 SB on/off & Osborne	0.878	0.932
19. Laurel Canyon & Sheldon	0.740	0.765
Level of service ranges:	.00 - .60 A	
	.61 - .70 B	
	.71 - .80 C	
	.81 - .90 D	
	.91 – 1.00 E	
	Above 1.00 F	

Chapter 3.0

TRAFFIC IMPACT ANALYSIS

This chapter describes the potential impacts of the proposed development upon the surrounding arterial network. Traffic generated by development of the proposed project is added to the existing volumes presented in the previous chapter and the resulting capacity impacts are assessed.

3.1 TRIP GENERATION

As discussed in Chapter 1.0, LADWP is proposing modifications to a power generating station to help comply with its annual Regional Clean Air Incentives Market (RECLAIM) Allocations for future years, improve in-Basin power reliability, and participate in the Californian Independent System Operator (“Cal-ISO”) by supplying excess electrical power thereby reducing the risk of blackouts for the state. The proposed project is anticipated to provide an overall decrease in No_x throughout the Basin. The proposed project consists of modifications to the Valley Generating Station (VGS) located in Sun Valley.

The following table summarizes the anticipated peak construction workers at the generating station.

CONSTRUCTION WORKER SUMMARY

PHASE	# WORKERS	ESTIMATED CONSTRUCTION TIME
1. Excavation	400	6 months
2. Foundation	100	11 months
3. Equipment Installation (there will be overlap among phases)		10 months
CTS		9 months
Steam Turbine		10 months
HRSGS		
4. Auxiliary Equipment	100	18 months
Source: ENSR		

The construction effort is anticipated to require 600 daily workers per day during the peak construction period. This peak construction period is anticipated to last for six months. This analysis uses a 1.0 vehicle occupancy for the construction workers to provide a "worst-case" analysis. These 600 peak daily construction worker vehicles will be arriving and departing during a single shift beginning at 6:00 AM and ending at 5:00 PM, resulting in 1,200 daily vehicle trips.

The morning AM peak hour of the adjacent street system occurs during the morning AM peak period of 7:00 AM to 9:00 AM as indicated in the Congestion Management Program (CMP) Guidelines. Construction activities at the VGS will occur six days a week. The workshift is scheduled to begin at 6:00 AM and end at 5:00 PM. Traffic attributable to the project construction traffic will arrive at the site before the morning AM peak period would begin and will not affect the morning AM peak hour ICU values. Traffic for the workshift will leave at the beginning of the afternoon PM peak period and may affect the afternoon PM peak hour ICU values. Therefore, the analysis examines impacts from traffic attributable to the proposed project only during the afternoon PM peak hour.

3.2 TRIP DISTRIBUTION

Distribution of project generated traffic was derived from observation of existing travel patterns in the vicinity of the project site. An increase in vehicular movements will occur at the project site during the construction period. The anticipated construction traffic at the VGS is forecast to peak at 600 vehicles per day.

To provide a "worst-case" analysis, it is assumed that most of the construction personnel would commute to the site in private automobiles.

Materials required to support the construction effort would be delivered to the generating station by truck. Peak truck usage would correspond to the peak manpower periods. Construction materials, heavy construction equipment, piping, and new equipment would be delivered throughout the construction period. All truck deliveries would be made at the main entrance from Sheldon Street.

To estimate the project-related traffic volumes at various points on the transportation system adjacent to the generating station and thereby establish the magnitude and extent of traffic impacts, a three-step process was utilized. First, the amount of traffic which would be generated during construction was determined. Second, the construction traffic was geographically distributed to appropriate

residential, commercial, and industrial areas. Finally, the trips were assigned to specific roadways and the traffic increases were evaluated on a route-by-route basis.

The average daily truck traffic at the VGS during construction is estimate to be approximately 70 trucks per day. Since these would mainly consist of soil and material deliveries, they would be spread throughout the work day with few deliveries occurring during the traffic analysis morning or afternoon peak hours (7:00am-9:00am and 4:00pm-6:00pm). Therefore, the truck traffic contribution from the proposed project to peak hour traffic analysis impacts would be negligible. As a conservative or "worst case" analysis, the maximum expected employees (600 employees) at the construction site was assumed to occur daily.

3.3 2001/EXISTING PLUS PROJECT TRAFFIC IMPACTS

The VGS modification project would generate short-term impacts on traffic and circulation in the project vicinity during the construction period. The project would temporarily affect the present pattern of circulation of the labor force as well as truck traffic associated with the construction and operation phases of the project.

Construction traffic related to the proposed project would utilize existing parking areas at the facility during construction. It would not affect the existing facility operations or the shipping and receiving facilities at the proposed project site.

Trip distribution for project traffic is illustrated in Figure 3-1 and is distributed to the surrounding roadways with thirty-five percent directed northward along the Golden State Freeway (I-5), twelve percent eastward toward the Foothill Freeway (SR-210), twenty-five percent directed southward via the SR-170 Freeway and the remaining twenty-eight percent directed south/southeast of the site along the Golden State Freeway (I-5).

Roadways in the vicinity of the project would be impacted by the project's construction-related traffic. However, project related construction traffic would contribute less than two percent of the daily traffic volume on these roadways.

Figure 3-1

To more carefully assess the impacts on the surrounding roadways, an intersection capacity utilization (ICU) analysis was conducted for the 19 intersections which would be most directly impacted by project construction traffic.

Analysis year-plus-project intersection volumes for the project were generated by adding the project intersection volumes to the existing Year 2001 background intersection volumes. PM peak hour 2001-plus-project turn volumes are illustrated in Figure 3-2, and corresponding ICUs based on existing lane configurations are summarized in the following Table 3-1 (actual ICU calculations are included in Appendix A). An examination of this table reveals that project construction traffic does have an impact considered significant by SCAQMD criteria at one location (San Fernando and Sheldon). The intersection of San Fernando and Sheldon exceeds the acceptable LOS 'E' with the addition of project related traffic and it therefore requires to mitigation.

3.4 POTENTIAL MITIGATION

Potential mitigation examined for the proposed project was a rideshare program to achieve a 1.3 vehicle occupancy for the construction workers. The 1.3 V/O target threshold is based on construction contractor experience from Fluor-Daniel regarding average vehicle ridership at similar construction projects (i.e., LARC and LARW). This results in a forecast of 462 construction worker vehicles entering and exiting the site during the peak six month construction period ($600 \text{ workers} / 1.3 \text{ workers/vehicle} = 462 \text{ vehicles}$). These 462 peak daily construction worker vehicles would be arriving and departing during a single shift beginning at 6:00 AM and ending at 5:00 PM, resulting in 924 daily vehicle trips. The results of implementation of this mitigation on impacted study area intersections is summarized in Table 3-1.

An examination of Table 3-1 indicates that the proposed mitigation measures result in study area intersections operating at an acceptable level of service with the addition of project traffic under existing-plus-project conditions.

figure 3-2

3.5 ON-SITE CIRCULATION AND PARKING

Sufficient on-site parking is available to accommodate the increased parking demand from construction workers at the Valley Generation Station. The physical site of the VGS provides parking capacity beyond the current operational requirements. On any given day, approximately 25 percent of the employees are not on the premises because of rotating shifts, vacations, and sick leave. The total number of parking spaces exceeds the maximum number of construction workers to allow for fluctuations in manpower and to provide ample maneuvering space for heavy trucks.

Table 3-1

ICU SUMMARY – EXISTING PLUS PROJECT CONDITIONS

INTERSECTION	EXISTING	EXISTING + PROJECT		EXISTING + PROJECT WITH MITIGATION	
	PM	PM	% CHG	PM	% CHG
1. Glenoaks & Sheldon	0.463	0.463	NC	0.463	NC
2. Glenoaks & Tuxford	0.646	0.646	NC	0.646	NC
3. Sunland & Glenoaks	0.697	0.697	NC	0.697	NC
4. San Fernando & Osborne	0.799	0.799	NC	0.799	NC
5. San Fernando & Sheldon	0.800	0.922	.122 ⁽¹⁾	0.893	.093
6. Arleta & Sheldon	0.754	0.760	.006	0.759	.005
7. Coldwater Canyon & Roscoe	1.106	1.106	NC	1.106	NC
8. SR-170 SB Ramp & Roscoe	1.206	1.206	NC	1.206	NC
9. SR-170 NB Ramp & Roscoe	0.888	0.888	NC	0.888	NC
10. SR-170 NB off & Sheldon	0.509	0.509	NC	0.509	NC
11. SR-170 SB ramp & Arleta	0.749	0.790	.041	0.781	.032
12. Sunland & I-5 NB on/off	0.594	0.594	NC	0.594	NC
13. Sunland & I-5 SB on/off	0.848	0.848	NC	0.848	NC
14. I-5 NB on/Rincon & Sheldon	0.575	0.575	NC	0.575	NC
15. Laurel Canyon & I-5 SB on/off	0.656	0.686	.030	0.679	.023
16. Laurel Canyon & I-5 NB off	0.500	0.500	NC	0.500	NC
17. I-5 NB on/off & Osborne	0.704	0.704	NC	0.704	NC
18. I-5 SB on/off & Osborne	0.932	0.932	NC	0.932	NC
19. Laurel Canyon & Sheldon	0.765	0.812	.047	0.801	.036

⁽¹⁾Significant Impact based on SCAQMD

Level of service ranges: .00 - .60 A
.61 - .70 B
.71 - .80 C
.81 - .90 D
.91 - 1.00 E
Above 1.00 F

Chapter 4.0

MITIGATION MEASURES

This chapter addresses the capacity deficiencies identified in the project impact analysis presented in the previous chapter.

Project construction traffic does have an impact considered significant by SCAQMD guidelines on the forecast PM peak hour level of service at one study area location (San Fernando and Sheldon) and is therefore subject to mitigation.

Recommended mitigation during the construction phase of the project is the requirement for a rideshare program to achieve a target threshold vehicle occupancy of 1.3 persons per vehicle.

No mitigation measures are needed for the small increase in truck traffic to and from the generating station related to the transportation of aqueous ammonia and materials required for reducing Nitrogen (NO_x) emissions.

Adequate off-street parking inside the generating station will be provided to accommodate the peak construction and operating labor force after completion of the project.

The entry point to the generating station for construction, commuter and delivery vehicles minimizes impacts on traffic and circulation patterns on the street system near the generating station, and maintains access for pedestrians, bicyclists, and motor vehicle traffic.

Truck operations will disperse deliveries throughout the off-peak hours to minimize peak hour traffic impacts.

If required, truck operations for the delivery of over-size equipment and materials will be conducted to the maximum extent possible during off-peak hours to minimize traffic impacts.

APPENDIX A
INTERSECTION CAPACITY UTILIZATION

INTERSECTION CAPACITY UTILIZATION

Intersection: Glenoaks Blvd & Sheldon St

AM Count Date: 4/25/01

PM Count Date: 4/24/01

AM Peak Hour: 7:15A-8:15A

PM Peak Hour: 4:30P-5:30P

Analyst: TDS

Agency: Los Angeles

Hour	V/C	Movement	No. of Lanes	Capacity	AM Peak Hour Volume	V/C	PM Peak Volume
.048		NB Left	1	1600	77	.048*	76
.137*		NB Thru	3	4800	357	.081	650
		NB Right	0	0	33		7
.026*		SB Left	1	1600	58	.036	42
.086		SB Thru	3	4800	742	.194*	350
		SB Right	0	0	190		63
.156*		EB Left	1	1600	197	.123*	249
.132		EB Thru	2	3200	173	.083	372
		EB Right	0	0	91		51
.023		WB Left	1	1600	44	.028	36
.044*		WB Thru	2	3200	397	.152*	94
		WB Right	0	0	89		47

.363	Sum of Critical V/C Ratios (*)	.517
.100	Adjustment for Lost Time	.100
.463	INTERSECTION CAPACITY UTILIZATION	.617
A	Level of Service	B

	LOS	Maximum
ICU		-----
.60	A	
.70	B	
.80	C	
.90	D	
1.00	E	
n/a	F	

INTERSECTION CAPACITY UTILIZATION

Intersection: Glenoaks Blvd & Sheldon St

AM Count Date: Exist+Prj

PM Count Date: Exist+Prj

AM Peak Hour: 7:15A-8:15A

PM Peak Hour: 4:30P-5:30P

Analyst: TDS

Agency: Los Angeles

Hour	V/C	Movement	No. of Lanes	Capacity	AM Peak Hour Volume	V/C	PM Peak Volume
.048		NB Left	1	1600	77	.048*	76
.137*		NB Thru	3	4800	357	.081	650
		NB Right	0	0	33		7
.026*		SB Left	1	1600	58	.036	42
.086		SB Thru	3	4800	742	.194*	350
		SB Right	0	0	190		63
.156*		EB Left	1	1600	197	.123*	249
.173		EB Thru	2	3200	173	.083	444
		EB Right	0	0	91		111
.023		WB Left	1	1600	44	.028	36
.044*		WB Thru	2	3200	397	.152*	94
		WB Right	0	0	89		47

.363	Sum of Critical V/C Ratios (*)	.517
.100	Adjustment for Lost Time	.100
INTERSECTION CAPACITY UTILIZATION	.617	.463
A	Level of Service	B

ICU	LOS	Maximum
-----	-----	-----
.60	A	
.70	B	
.80	C	
.90	D	
1.00	E	
n/a	F	

INTERSECTION CAPACITY UTILIZATION

Intersection: Glenoaks Blvd & Sheldon St

AM Count Date: Exist+Prj w/Mit

PM Count Date: Exist+Prj w/Mit

AM Peak Hour: 7:15A-8:15A

PM Peak Hour: 4:30P-5:30P

Analyst: TDS

Agency: Los Angeles

Hour	V/C	Movement	No. of Lanes	Capacity	AM Peak Hour Volume	V/C	PM Peak Volume
.048		NB Left	1	1600	77	.048*	76
.137*		NB Thru	3	4800	357	.081	650
		NB Right	0	0	33		7
.026*		SB Left	1	1600	58	.036	42
.086		SB Thru	3	4800	742	.194*	350
		SB Right	0	0	190		63
.156*		EB Left	1	1600	197	.123*	249
.164		EB Thru	2	3200	173	.083	427
		EB Right	0	0	91		97
.023		WB Left	1	1600	44	.028	36
.044*		WB Thru	2	3200	397	.152*	94
		WB Right	0	0	89		47

.363	Sum of Critical V/C Ratios (*)	.517
.100	Adjustment for Lost Time	.100
.463	INTERSECTION CAPACITY UTILIZATION	.617
A	Level of Service	B

	LOS	Maximum
ICU		-----
.60	A	
.70	B	
.80	C	
.90	D	
1.00	E	
n/a	F	

INTERSECTION CAPACITY UTILIZATION

Intersection: Glenoaks Blvd & Tuxford St

AM Count Date: 4/30/01

PM Count Date: 4/26/01

AM Peak Hour: 7:15A-8:15A

PM Peak Hour: 4:45P-5:45P

Analyst: TDS

Agency: Los Angeles

Hour	V/C	Movement	No. of Lanes	Capacity	AM Peak Hour Volume	V/C	PM Peak Volume
.069	.213*	NB Left	1	1600	98	.061*	110
		NB Thru	2	3200	450	.161	637
		NB Right	0	0	64		44
.093*	.199	SB Left	1	1600	123	.077	148
		SB Thru	2	3200	540	.197*	521
		SB Right	0	0	89		116
.077	.225*	EB Left	1	1600	125	.078*	123
		EB Thru	2	3200	394	.143	595
		EB Right	0	0	62		126
.015*	.157	WB Left	1	1600	83	.052	24
		WB Thru	2	3200	645	.234*	389
		WB Right	0	0	104		112

.546	Sum of Critical V/C Ratios (*)	.570
.100	Adjustment for Lost Time	.100
.646	INTERSECTION CAPACITY UTILIZATION	.670
B	Level of Service	B

ICU	LOS	Maximum
-----		-----
.60	A	
.70	B	
.80	C	
.90	D	
1.00	E	
n/a	F	

INTERSECTION CAPACITY UTILIZATION

Intersection: Glenoaks Blvd & Tuxford St

AM Count Date: Exist+Prj

PM Count Date: Exist+Prj

AM Peak Hour: 7:15A-8:15A

PM Peak Hour: 4:45P-5:45P

Analyst: TDS

Agency: Los Angeles

Hour	V/C	Movement	No. of Lanes	Capacity	AM Peak Hour Volume	V/C	PM Peak Volume
.069		NB Left	1	1600	98	.061*	110
.213*		NB Thru	2	3200	450	.161	637
		NB Right	0	0	64		44
.093*		SB Left	1	1600	123	.077	148
.218		SB Thru	2	3200	540	.197*	521
		SB Right	0	0	89		176
.077		EB Left	1	1600	125	.078*	123
.225*		EB Thru	2	3200	394	.143	595
		EB Right	0	0	62		126
.015*		WB Left	1	1600	83	.052	24
.157		WB Thru	2	3200	645	.234*	389
		WB Right	0	0	104		112

.546	Sum of Critical V/C Ratios (*)	.570
.100	Adjustment for Lost Time	.100
.646	INTERSECTION CAPACITY UTILIZATION	.670
B	Level of Service	B

ICU	LOS	Maximum
-----		-----
.60	A	
.70	B	
.80	C	
.90	D	
1.00	E	
n/a	F	

INTERSECTION CAPACITY UTILIZATION

Intersection: Glenoaks Blvd & Tuxford St

AM Count Date: Exist+Prj w/Mit

PM Count Date: Exist+Prj w/Mit

AM Peak Hour: 7:15A-8:15A

PM Peak Hour: 4:45P-5:45P

Analyst: TDS

Agency: Los Angeles

Hour	V/C	Movement	No. of Lanes	Capacity	AM Peak Hour Volume	V/C	PM Peak Volume
.069	.213*	NB Left	1	1600	98	.061*	110
		NB Thru	2	3200	450	.161	637
		NB Right	0	0	64		44
.093*	.213	SB Left	1	1600	123	.077	148
		SB Thru	2	3200	540	.197*	521
		SB Right	0	0	89		162
.077	.225*	EB Left	1	1600	125	.078*	123
		EB Thru	2	3200	394	.143	595
		EB Right	0	0	62		126
.015*	.157	WB Left	1	1600	83	.052	24
		WB Thru	2	3200	645	.234*	389
		WB Right	0	0	104		112

.546	Sum of Critical V/C Ratios (*)	.570
.100	Adjustment for Lost Time	.100
.646	INTERSECTION CAPACITY UTILIZATION	.670
B	Level of Service	B

ICU	LOS	Maximum
-----		-----
.60	A	
.70	B	
.80	C	
.90	D	
1.00	E	
n/a	F	

INTERSECTION CAPACITY UTILIZATION

Intersection: Sunland Blvd & Glenoaks Blvd

AM Count Date: 4/25/01

PM Count Date: 4/25/01

AM Peak Hour: 7:30A-8:30A

PM Peak Hour: 5:00P-6:00P

Analyst: TDS

Agency: Los Angeles

Hour	V/C	Movement	No. of Lanes	Capacity	AM Peak Hour Volume	V/C	PM Peak Volume
.052		NB Left	1	1600	120	.075	83
.186*		NB Thru	2	3200	464	.145*	575
		NB Right	0	0	0		19
.177*		SB Left	1	1600	589	.368*	283
.158		SB Thru	2	3200	890	.280	484
		SB Right	0	0	5		22
.023*		EB Left	1	1600	17	.011	36
.123		EB Thru	2	3200	552	.215*	370
		EB Right	0	0	136		22
.021		WB Left	1	1600	10	.006*	34
.211*		WB Thru	2	3200	350	.179	388
		WB Right	0	0	222		287

.597	Sum of Critical V/C Ratios (*)	.734
.100	Adjustment for Lost Time	.100
.697	INTERSECTION CAPACITY UTILIZATION	.834
B	Level of Service	D

ICU	LOS	Maximum
-----		-----
.60	A	
.70	B	
.80	C	
.90	D	
1.00	E	
n/a	F	

INTERSECTION CAPACITY UTILIZATION

Intersection: Sunland Blvd & Glenoaks Blvd

AM Count Date: Exist+Prj

PM Count Date: Exist+Prj

AM Peak Hour: 7:30A-8:30A

PM Peak Hour: 5:00P-6:00P

Analyst: TDS

Agency: Los Angeles

Hour	V/C	Movement	No. of Lanes	Capacity	AM Peak Hour Volume	V/C	PM Peak Volume
.052		NB Left	1	1600	120	.075	83
.186*		NB Thru	2	3200	464	.145*	575
		NB Right	0	0	0		19
.177*		SB Left	1	1600	589	.368*	283
.158		SB Thru	2	3200	890	.280	484
		SB Right	0	0	5		22
.023*		EB Left	1	1600	17	.011	36
.123		EB Thru	2	3200	552	.215*	370
		EB Right	0	0	136		22
.021		WB Left	1	1600	10	.006*	34
.211*		WB Thru	2	3200	350	.179	388
		WB Right	0	0	222		287

.597	Sum of Critical V/C Ratios (*)	.734
.100	Adjustment for Lost Time	.100
.697	INTERSECTION CAPACITY UTILIZATION	.834
B	Level of Service	D

	LOS	Maximum
ICU		-----
.60	A	
.70	B	
.80	C	
.90	D	
1.00	E	
n/a	F	

INTERSECTION CAPACITY UTILIZATION

Intersection: Sunland Blvd & Glenoaks Blvd

AM Count Date: Exist+Prj w/Mit

PM Count Date: Exist+Prj w/Mit

AM Peak Hour: 7:30A-8:30A

PM Peak Hour: 5:00P-6:00P

Analyst: TDS

Agency: Los Angeles

Hour	V/C	Movement	No. of Lanes	Capacity	AM Peak Hour Volume	V/C	PM Peak Volume
.052		NB Left	1	1600	120	.075	83
.186*		NB Thru	2	3200	464	.145*	575
		NB Right	0	0	0		19
.177*		SB Left	1	1600	589	.368*	283
.158		SB Thru	2	3200	890	.280	484
		SB Right	0	0	5		22
.023*		EB Left	1	1600	17	.011	36
.123		EB Thru	2	3200	552	.215*	370
		EB Right	0	0	136		22
.021		WB Left	1	1600	10	.006*	34
.211*		WB Thru	2	3200	350	.179	388
		WB Right	0	0	222		287

.597	Sum of Critical V/C Ratios (*)	.734
.100	Adjustment for Lost Time	.100
.697	INTERSECTION CAPACITY UTILIZATION	.834
B	Level of Service	D

	LOS	Maximum
ICU		-----
.60	A	
.70	B	
.80	C	
.90	D	
1.00	E	
n/a	F	

INTERSECTION CAPACITY UTILIZATION

Intersection: San Fernando & Osborne St

AM Count Date: 4/24/01

PM Count Date: 4/25/01

AM Peak Hour: 7:00A-8:00A

PM Peak Hour: 4:45P-5:45P

Analyst: TDS

Agency: Los Angeles

Hour	V/C	Movement	No. of Lanes	Capacity	AM Peak Hour Volume	AM Peak Hour V/C	PM Peak Hour Volume
.057	.324*	NB Left	1	1600	46	.029	91
		NB Thru	2	3200	414	.159*	873
		NB Right	0	0	94		164
.089*	.163	SB Left	1	1600	229	.143*	143
		SB Thru	2	3200	721	.240	420
		SB Right	0	0	48		103
.041*	.180	EB Left	1	1600	107	.067*	65
		EB Thru	2	3200	612	.208	552
		EB Right	0	0	52		25
.043	.245*	WB Left	1	1600	163	.102	68
		WB Thru	2	3200	805	.279*	617
		WB Right	0	0	89		168

.699	Sum of Critical V/C Ratios (*)	.648
.100	Adjustment for Lost Time	.100
INTERSECTION CAPACITY UTILIZATION	.748	.799
C	Level of Service	C

ICU	LOS	Maximum
-----	-----	-----
.60	A	
.70	B	
.80	C	
.90	D	
1.00	E	
n/a	F	

INTERSECTION CAPACITY UTILIZATION

Intersection: San Fernando & Osborne St

AM Count Date: Exist+Prj

PM Count Date: Exist+Prj

AM Peak Hour: 7:00A-8:00A

PM Peak Hour: 4:45P-5:45P

Analyst: TDS

Agency: Los Angeles

Hour	V/C	Movement	No. of Lanes	Capacity	AM Peak Hour Volume	V/C	PM Peak Volume
.151		NB Left	1	1600	46	.029	241
.324*		NB Thru	2	3200	414	.159*	873
		NB Right	0	0	94		164
.089*		SB Left	1	1600	229	.143*	143
.163		SB Thru	2	3200	721	.240	420
		SB Right	0	0	48		103
.041*		EB Left	1	1600	107	.067*	65
.180		EB Thru	2	3200	612	.208	552
		EB Right	0	0	52		25
.043		WB Left	1	1600	163	.102	68
.245*		WB Thru	2	3200	805	.279*	617
		WB Right	0	0	89		168

.699	Sum of Critical V/C Ratios (*)	.648
.100	Adjustment for Lost Time	.100
INTERSECTION CAPACITY UTILIZATION	.748	.799
C	Level of Service	C

ICU	LOS	Maximum
-----	-----	-----
.60	A	
.70	B	
.80	C	
.90	D	
1.00	E	
n/a	F	

INTERSECTION CAPACITY UTILIZATION

Intersection: San Fernando & Osborne St

AM Count Date: Exist+Prj w/Mit

PM Count Date: Exist+Prj w/Mit

AM Peak Hour: 7:00A-8:00A

PM Peak Hour: 4:45P-5:45P

Analyst: TDS

Agency: Los Angeles

Hour	V/C	Movement	No. of Lanes	Capacity	AM Peak Hour Volume	V/C	PM Peak Volume
.129		NB Left	1	1600	46	.029	207
.324*		NB Thru	2	3200	414	.159*	873
		NB Right	0	0	94		164
.089*		SB Left	1	1600	229	.143*	143
.163		SB Thru	2	3200	721	.240	420
		SB Right	0	0	48		103
.041*		EB Left	1	1600	107	.067*	65
.180		EB Thru	2	3200	612	.208	552
		EB Right	0	0	52		25
.043		WB Left	1	1600	163	.102	68
.245*		WB Thru	2	3200	805	.279*	617
		WB Right	0	0	89		168

.699	Sum of Critical V/C Ratios (*)	.648
.100	Adjustment for Lost Time	.100
.799	INTERSECTION CAPACITY UTILIZATION	.748
C	Level of Service	C

	LOS	Maximum
ICU		-----
.60	A	
.70	B	
.80	C	
.90	D	
1.00	E	
n/a	F	

INTERSECTION CAPACITY UTILIZATION

Intersection: San Fernando & Sheldon St

AM Count Date: 4/25/01

PM Count Date: 4/25/01

AM Peak Hour: 7:15A-8:15A

PM Peak Hour: 4:30P-5:30P

Analyst: TDS

Agency: Los Angeles

Hour	V/C	Movement	No. of Lanes	Capacity	AM Peak Hour Volume	V/C	PM Peak Volume
.070	.319*	NB Left	1	1600	58	.036	112
		NB Thru	2	3200	435	.189*	866
		NB Right	0	0	169		155
.055*	.224	SB Left	1	1600	134	.084*	88
		SB Thru	2	3200	644	.223	610
		SB Right	0	0	68		108
.114	.227*	EB Left	1	1600	103	.064	182
		EB Thru	2	3200	501	.166*	683
		EB Right	0	0	31		45
.099*	.206	WB Left	1	1600	144	.090*	159
		WB Thru	2	3200	570	.191	576
		WB Right	0	0	41		84

.700	Sum of Critical V/C Ratios (*)	.529
.100	Adjustment for Lost Time	.100
.800	INTERSECTION CAPACITY UTILIZATION	.629
C	Level of Service	B

ICU	LOS	Maximum
.60	A	
.70	B	
.80	C	
.90	D	
1.00	E	
n/a	F	

INTERSECTION CAPACITY UTILIZATION

Intersection: San Fernando & Sheldon St

AM Count Date: Exist+Prj

PM Count Date: Exist+Prj

AM Peak Hour: 7:15A-8:15A

PM Peak Hour: 4:30P-5:30P

Analyst: TDS

Agency: Los Angeles

Hour	V/C	Movement	No. of Lanes	Capacity	AM Peak Hour Volume	AM Peak Hour V/C	PM Peak Volume
.070	.319*	NB Left	1	1600	58	.036	112
		NB Thru	2	3200	435	.189*	866
		NB Right	0	0	169		155
.055*	.224	SB Left	1	1600	134	.084*	88
		SB Thru	2	3200	644	.223	610
		SB Right	0	0	68		108
.114*	.227	EB Left	1	1600	103	.064	182
		EB Thru	2	3200	501	.166*	683
		EB Right	0	0	31		45
.137	.334*	WB Left	1	1600	144	.090*	219
		WB Thru	2	3200	570	.191	834
		WB Right	0	0	41		234

.822	Sum of Critical V/C Ratios (*)	.529
.100	Adjustment for Lost Time	.100
.922	INTERSECTION CAPACITY UTILIZATION	.629
E	Level of Service	B

ICU	LOS	Maximum
.60	A	
.70	B	
.80	C	
.90	D	
1.00	E	
n/a	F	

INTERSECTION CAPACITY UTILIZATION

Intersection: San Fernando & Sheldon St

AM Count Date: Exist+Prj w/Mit

PM Count Date: Exist+Prj w/Mit

AM Peak Hour: 7:15A-8:15A

PM Peak Hour: 4:30P-5:30P

Analyst: TDS

Agency: Los Angeles

Hour	V/C	Movement	No. of Lanes	Capacity	AM Peak Hour Volume	V/C	PM Peak Volume
.070		NB Left	1	1600	58	.036	112
.319*		NB Thru	2	3200	435	.189*	866
		NB Right	0	0	169		155
.055*		SB Left	1	1600	134	.084*	88
.224		SB Thru	2	3200	644	.223	610
		SB Right	0	0	68		108
.114*		EB Left	1	1600	103	.064	182
.227		EB Thru	2	3200	501	.166*	683
		EB Right	0	0	31		45
.128		WB Left	1	1600	144	.090*	205
.305*		WB Thru	2	3200	570	.191	775
		WB Right	0	0	41		200

.793	Sum of Critical V/C Ratios (*)	.529
.100	Adjustment for Lost Time	.100
INTERSECTION CAPACITY UTILIZATION	.629	.893
D	Level of Service	B

ICU	LOS	Maximum
-----	-----	-----
.60	A	
.70	B	
.80	C	
.90	D	
1.00	E	
n/a	F	

INTERSECTION CAPACITY UTILIZATION

Intersection: Arleta Ave & Sheldon St

AM Count Date: 4/24/01

PM Count Date: 4/25/01

AM Peak Hour: 7:30A-8:30A

PM Peak Hour: 4:30P-5:30P

Analyst: TDS

Agency: Los Angeles

Hour	V/C	Movement	No. of Lanes	Capacity	AM Peak Hour Volume	AM Peak Hour V/C	PM Peak Volume
.008	.268*	NB Left	1	1600	9	.006*	12
		NB Thru	2	3200	112	.040	760
		NB Right	0	0	17		97
.057*	.119	SB Left	1	1600	97	.061	91
		SB Thru	2	3200	367	.178*	302
		SB Right	0	0	204		78
.213*	.198	EB Left	1	1600	75	.047*	340
		EB Thru	2	3200	277	.091	628
		EB Right	0	0	15		7
.027	.116*	WB Left	1	1600	51	.032	43
		WB Thru	2	3200	508	.159*	372
		WB Right	f		247		607

.654	Sum of Critical V/C Ratios (*)	.390
.100	Adjustment for Lost Time	.100
.754	INTERSECTION CAPACITY UTILIZATION	.490
C	Level of Service	A

	LOS	Maximum
ICU		-----
.60	A	
.70	B	
.80	C	
.90	D	
1.00	E	
n/a	F	

INTERSECTION CAPACITY UTILIZATION

Intersection: Arleta Ave & Sheldon St

AM Count Date: Exist+Prj

PM Count Date: Exist+Prj

AM Peak Hour: 7:30A-8:30A

PM Peak Hour: 4:30P-5:30P

Analyst: TDS

Agency: Los Angeles

Hour	V/C	Movement	No. of Lanes	Capacity	AM Peak Hour Volume	V/C	PM Peak Volume
.008	.268*	NB Left	1	1600	9	.006*	12
		NB Thru	2	3200	112	.040	760
		NB Right	0	0	17		97
.057*	.119	SB Left	1	1600	97	.061	91
		SB Thru	2	3200	367	.178*	302
		SB Right	0	0	204		78
.213*	.198	EB Left	1	1600	75	.047*	340
		EB Thru	2	3200	277	.091	628
		EB Right	0	0	15		7
.027	.122*	WB Left	1	1600	51	.032	43
		WB Thru	2	3200	508	.159*	390
		WB Right	f		247		739

.660	Sum of Critical V/C Ratios (*)	.390
.100	Adjustment for Lost Time	.100
.760	INTERSECTION CAPACITY UTILIZATION	.490
C	Level of Service	A

ICU	LOS	Maximum
.60	A	
.70	B	
.80	C	
.90	D	
1.00	E	
n/a	F	

INTERSECTION CAPACITY UTILIZATION

Intersection: Arleta Ave & Sheldon St

AM Count Date: Exist+Prj w/Mit

PM Count Date: Exist+Prj w/Mit

AM Peak Hour: 7:30A-8:30A

PM Peak Hour: 4:30P-5:30P

Analyst: TDS

Agency: Los Angeles

Hour	V/C	Movement	No. of Lanes	Capacity	AM Peak Hour Volume	V/C	PM Peak Volume
.008	.268*	NB Left	1	1600	9	.006*	12
		NB Thru	2	3200	112	.040	760
		NB Right	0	0	17		97
.057*	.119	SB Left	1	1600	97	.061	91
		SB Thru	2	3200	367	.178*	302
		SB Right	0	0	204		78
.213*	.198	EB Left	1	1600	75	.047*	340
		EB Thru	2	3200	277	.091	628
		EB Right	0	0	15		7
.027	.121*	WB Left	1	1600	51	.032	43
		WB Thru	2	3200	508	.159*	386
		WB Right	f		247		709

.659	Sum of Critical V/C Ratios (*)	.390
.100	Adjustment for Lost Time	.100
.759	INTERSECTION CAPACITY UTILIZATION	.490
C	Level of Service	A

ICU	LOS	Maximum
.60	A	
.70	B	
.80	C	
.90	D	
1.00	E	
n/a	F	

INTERSECTION CAPACITY UTILIZATION

Intersection: Coldwater Cyn & Roscoe

AM Count Date: 4/25/01

PM Count Date: 4/24/01

AM Peak Hour: 7:15A-8:15A

PM Peak Hour: 4:30P-5:30P

Analyst: TDS

Agency: Los Angeles

Hour	V/C	Movement	No. of Lanes	Capacity	AM Peak Hour Volume	V/C	PM Peak Volume
.156	.325*	NB Left	1	1600	135	.084*	249
		NB Thru	2	3200	370	.219	735
		NB Right	0	0	331		304
.026*	.153	SB Left	1	1600	12	.008	41
		SB Thru	2	3200	641	.268*	359
		SB Right	0	0	217		130
.256	.404*	EB Left	1	1600	133	.083	410
		EB Thru	3	4800	1508	.349*	1807
		EB Right	0	0	169		130
.251*	.376	WB Left	1	1600	317	.198*	401
		WB Thru	3	4800	1430	.304	1725
		WB Right	0	0	28		80

1.006	Sum of Critical V/C Ratios (*)	.899
.100	Adjustment for Lost Time	.100
INTERSECTION CAPACITY UTILIZATION	.999	1.106
F	Level of Service	E

ICU	LOS	Maximum
-----	-----	-----
.60	A	
.70	B	
.80	C	
.90	D	
1.00	E	
n/a	F	

INTERSECTION CAPACITY UTILIZATION

Intersection: Coldwater Cyn & Roscoe

AM Count Date: Exist+Prj

PM Count Date: Exist+Prj

AM Peak Hour: 7:15A-8:15A

PM Peak Hour: 4:30P-5:30P

Analyst: TDS

Agency: Los Angeles

Hour	V/C	Movement	No. of Lanes	Capacity	AM Peak Hour Volume	V/C	PM Peak Volume
.156	.325*	NB Left	1	1600	135	.084*	249
		NB Thru	2	3200	370	.219	735
		NB Right	0	0	331		304
.026*	.153	SB Left	1	1600	12	.008	41
		SB Thru	2	3200	641	.268*	359
		SB Right	0	0	217		130
.256	.404*	EB Left	1	1600	133	.083	410
		EB Thru	3	4800	1508	.349*	1807
		EB Right	0	0	169		130
.251*	.376	WB Left	1	1600	317	.198*	401
		WB Thru	3	4800	1430	.304	1725
		WB Right	0	0	28		80

1.006	Sum of Critical V/C Ratios (*)	.899
.100	Adjustment for Lost Time	.100
INTERSECTION CAPACITY UTILIZATION	.999	1.106
F	Level of Service	E

ICU	LOS	Maximum
-----	-----	-----
.60	A	
.70	B	
.80	C	
.90	D	
1.00	E	
n/a	F	

INTERSECTION CAPACITY UTILIZATION

Intersection: Coldwater Cyn & Roscoe

AM Count Date: Exist+Prj w/Mit

PM Count Date: Exist+Prj w/Mit

AM Peak Hour: 7:15A-8:15A

PM Peak Hour: 4:30P-5:30P

Analyst: TDS

Agency: Los Angeles

Hour	V/C	Movement	No. of Lanes	Capacity	AM Peak Hour Volume	V/C	PM Peak Volume
.156	.325*	NB Left	1	1600	135	.084*	249
		NB Thru	2	3200	370	.219	735
		NB Right	0	0	331		304
.026*	.153	SB Left	1	1600	12	.008	41
		SB Thru	2	3200	641	.268*	359
		SB Right	0	0	217		130
.256	.404*	EB Left	1	1600	133	.083	410
		EB Thru	3	4800	1508	.349*	1807
		EB Right	0	0	169		130
.251*	.376	WB Left	1	1600	317	.198*	401
		WB Thru	3	4800	1430	.304	1725
		WB Right	0	0	28		80

1.006	Sum of Critical V/C Ratios (*)	.899
.100	Adjustment for Lost Time	.100
1.106	INTERSECTION CAPACITY UTILIZATION	.999
F	Level of Service	E

	LOS	Maximum
ICU		-----
.60	A	
.70	B	
.80	C	
.90	D	
1.00	E	
n/a	F	

INTERSECTION CAPACITY UTILIZATION

Intersection: SR-170 NB ramp & Roscoe

AM Count Date: 4/25/01

PM Count Date: 4/24/01

AM Peak Hour: 7:15A-8:15A

PM Peak Hour: 4:30P-5:30P

Analyst: TDS

Agency: Los Angeles

Hour	V/C	Movement	No. of Lanes	Capacity	AM Peak Hour Volume	AM Peak Hour V/C	PM Peak Hour Volume
.659		NB Left	0	0	0		0
		NB Thru	0	0	0		0
		NB Right	1	1600	380	.238	1054
		SB Left	0	0	0		0
		SB Thru	0	0	0		0
		SB Right	f		775		1689
.431*		EB Left	0	0	0		0
		EB Thru	2	3200	917	.287*	1379
		EB Right	f		152		148
.064*		WB Left	1	1600	192	.120*	102
		WB Thru	2	3200	1140	.356	1550
		WB Right	0	0	0		0
.611*		Right Turn Adjustment			NBR	.148*	NBR

		LOS	Maximum
-----+			
1.106	Sum of Critical V/C Ratios (*)	.555	
.100	Adjustment for Lost Time	.100	
1.206	INTERSECTION CAPACITY UTILIZATION	.655	
F	Level of Service	B	
ICU			

.60		A	
.70		B	
.80		C	
.90		D	
1.00		E	
n/a		F	

INTERSECTION CAPACITY UTILIZATION

Intersection: SR-170 NB ramp & Roscoe

AM Count Date: Exist+Prj

PM Count Date: Exist+Prj

AM Peak Hour: 7:15A-8:15A

PM Peak Hour: 4:30P-5:30P

Analyst: TDS

Agency: Los Angeles

Hour	V/C	Movement	No. of Lanes	Capacity	AM Peak Hour Volume	V/C	PM Peak Volume
		NB Left	0	0	0		0
		NB Thru	0	0	0		0
		NB Right	1	1600	380	.238	1054
.659		SB Left	0	0	0		0
		SB Thru	0	0	0		0
		SB Right	f		775		1689
		EB Left	0	0	0		0
.431*		EB Thru	2	3200	917	.287*	1379
		EB Right	f		152		148
		WB Left	1	1600	192	.120*	102
.064*		WB Thru	2	3200	1140	.356	1550
.484		WB Right	0	0	0		0
		Right Turn Adjustment			NBR	.148*	NBR
.611*							

		LOS	Maximum
-----+			
1.106	Sum of Critical V/C Ratios (*)	.555	
.100	Adjustment for Lost Time	.100	
1.206	INTERSECTION CAPACITY UTILIZATION	.655	
F	Level of Service	B	
ICU			

.60		A	
.70		B	
.80		C	
.90		D	
1.00		E	
n/a		F	

INTERSECTION CAPACITY UTILIZATION

Intersection: SR-170 NB ramp & Roscoe

AM Count Date: Exist+Prj w/Mit

PM Count Date: Exist+Prj w/Mit

AM Peak Hour: 7:15A-8:15A

PM Peak Hour: 4:30P-5:30P

Analyst: TDS

Agency: Los Angeles

Hour	V/C	Movement	No. of Lanes	Capacity	AM Peak Hour Volume	V/C	PM Peak Volume
.659		NB Left	0	0	0		0
		NB Thru	0	0	0		0
		NB Right	1	1600	380	.238	1054
		SB Left	0	0	0		0
		SB Thru	0	0	0		0
		SB Right	f		775		1689
.431*		EB Left	0	0	0		0
		EB Thru	2	3200	917	.287*	1379
		EB Right	f		152		148
.064*		WB Left	1	1600	192	.120*	102
		WB Thru	2	3200	1140	.356	1550
		WB Right	0	0	0		0
.611*		Right Turn Adjustment			NBR	.148*	NBR

		+-----+	
-----+			
1.106	Sum of Critical V/C Ratios (*)	.555	
.100	Adjustment for Lost Time	.100	
INTERSECTION CAPACITY UTILIZATION	.655	1.206	
F	Level of Service	B	
ICU		LOS	Maximum
-----			-----
.60		A	
.70		B	
.80		C	
.90		D	
1.00		E	
n/a		F	

INTERSECTION CAPACITY UTILIZATION

Intersection: SR-170 SB ramp & Roscoe

AM Count Date: 4/25/01

PM Count Date: 4/24/01

AM Peak Hour: 7:15A-8:15A

PM Peak Hour: 4:30P-5:30P

Analyst: TDS

Agency: Los Angeles

Hour	V/C	Movement	No. of Lanes	Capacity	AM Peak Hour Volume	AM Peak Hour V/C	PM Peak Volume
		NB Left	0	0	0		0
		NB Thru	0	0	0		0
		NB Right	0	0	0		0
.175*		SB Left	1	1600	136	.085*	280
		SB Thru	0	0	0		0
.246		SB Right	1	1600	564	.353	394
		EB Left	0	0	0		0
.374		EB Thru	3	4800	1635	.341	1794
		EB Right	0	0	0		0
		WB Left	0	0	0		0
.542*		WB Thru	3	4800	1537	.389*	2177
		WB Right	0	0	332		423
.071*		Right Turn Adjustment			SBR	.268*	SBR

INTERSECTION CAPACITY UTILIZATION

Intersection: SR-170 SB ramp & Roscoe

AM Count Date: Exist+Prj

PM Count Date: Exist+Prj

AM Peak Hour: 7:15A-8:15A

PM Peak Hour: 4:30P-5:30P

Analyst: TDS

Agency: Los Angeles

Hour	V/C	Movement	No. of Lanes	Capacity	AM Peak Hour Volume	AM Peak Hour V/C	PM Peak Hour Volume
		NB Left	0	0	0		0
		NB Thru	0	0	0		0
		NB Right	0	0	0		0
		SB Left	1	1600	136	.085*	280
.175*		SB Thru	0	0	0		0
		SB Right	1	1600	564	.353	394
.246		EB Left	0	0	0		0
		EB Thru	3	4800	1635	.341	1794
.378		EB Right	0	0	0		18
		WB Left	0	0	0		0
		WB Thru	3	4800	1537	.389*	2177
.542*		WB Right	0	0	332		423
		Right Turn Adjustment			SBR	.268*	SBR
.071*							

		+-----+	
-----+			
.788	Sum of Critical V/C Ratios (*)	.742	
.100	Adjustment for Lost Time	.100	
INTERSECTION CAPACITY UTILIZATION	.842	.888	
D	Level of Service	D	
ICU		LOS	Maximum
-----			-----
.60		A	
.70		B	
.80		C	
.90		D	
1.00		E	
n/a		F	

INTERSECTION CAPACITY UTILIZATION

Intersection: SR-170 SB ramp & Roscoe

AM Count Date: Exist+Prj w/Mit

PM Count Date: Exist+Prj w/Mit

AM Peak Hour: 7:15A-8:15A

PM Peak Hour: 4:30P-5:30P

Analyst: TDS

Agency: Los Angeles

Hour	V/C	Movement	No. of Lanes	Capacity	AM Peak Hour Volume	AM Peak Hour V/C	PM Peak Hour Volume
		NB Left	0	0	0		0
		NB Thru	0	0	0		0
		NB Right	0	0	0		0
.175*		SB Left	1	1600	136	.085*	280
		SB Thru	0	0	0		0
.246		SB Right	1	1600	564	.353	394
		EB Left	0	0	0		0
.377		EB Thru	3	4800	1635	.341	1794
		EB Right	0	0	0		14
		WB Left	0	0	0		0
.542*		WB Thru	3	4800	1537	.389*	2177
		WB Right	0	0	332		423
.071*		Right Turn Adjustment			SBR	.268*	SBR

INTERSECTION CAPACITY UTILIZATION

Intersection: SR-170 NB off & Sheldon St

AM Count Date: 4/30/01

PM Count Date: 4/24/01

AM Peak Hour: 7:15A-8:15A

PM Peak Hour: 4:30P-5:30P

Analyst: TDS

Agency: Los Angeles

Hour	V/C	Movement	No. of Lanes	Capacity	AM Peak Hour Volume	AM Peak Hour V/C	PM Peak Hour Volume
		NB Left	0	0	0		0
		NB Thru	0	0	0		0
		NB Right	0	0	0		0
		SB Left	1.5		228		217
.136*		SB Thru	0	3200	0	.093*	0
		SB Right	0.5		71		223
.139		EB Left	0	0	0		0
		EB Thru	2	3200	656	.205	872
.273*		EB Right	0	0	0		0
		WB Left	0	0	0		0
		WB Thru	2	3200	1044	.326*	637
.199		WB Right	0	0	0		0

.409	Sum of Critical V/C Ratios (*)	.419
.100	Adjustment for Lost Time	.100
.509	INTERSECTION CAPACITY UTILIZATION	.519
A	Level of Service	A

	LOS	Maximum
ICU		-----
.60	A	
.70	B	
.80	C	
.90	D	
1.00	E	
n/a	F	

INTERSECTION CAPACITY UTILIZATION

Intersection: SR-170 NB off & Sheldon St

AM Count Date: Exist+Prj

PM Count Date: Exist+Prj

AM Peak Hour: 7:15A-8:15A

PM Peak Hour: 4:30P-5:30P

Analyst: TDS

Agency: Los Angeles

Hour	V/C	Movement	No. of Lanes	Capacity	AM Peak Hour Volume	V/C	PM Peak Volume
		NB Left	0	0	0		0
		NB Thru	0	0	0		0
		NB Right	0	0	0		0
		SB Left	1.5		228		217
.136*		SB Thru	0	3200	0	.093*	0
		SB Right	0.5		71		223
.139		EB Left	0	0	0		0
		EB Thru	2	3200	656	.205	872
.273*		EB Right	0	0	0		0
		WB Left	0	0	0		0
		WB Thru	2	3200	1044	.326*	787
.246		WB Right	0	0	0		0

.409	Sum of Critical V/C Ratios (*)	.419
.100	Adjustment for Lost Time	.100
.509	INTERSECTION CAPACITY UTILIZATION	.519
A	Level of Service	A

ICU	LOS	Maximum
-----	-----	-----
.60	A	
.70	B	
.80	C	
.90	D	
1.00	E	
n/a	F	

INTERSECTION CAPACITY UTILIZATION

Intersection: SR-170 NB off & Sheldon St

AM Count Date: Exist+Prj w/Mit

PM Count Date: Exist+Prj w/Mit

AM Peak Hour: 7:15A-8:15A

PM Peak Hour: 4:30P-5:30P

Analyst: TDS

Agency: Los Angeles

Hour	V/C	Movement	No. of Lanes	Capacity	AM Peak Hour Volume	AM Peak Hour V/C	PM Peak Hour Volume
		NB Left	0	0	0		0
		NB Thru	0	0	0		0
		NB Right	0	0	0		0
		SB Left	1.5		228		217
.136*		SB Thru	0	3200	0	.093*	0
		SB Right	0.5		71		223
.139		EB Left	0	0	0		0
		EB Thru	2	3200	656	.205	872
.273*		EB Right	0	0	0		0
		WB Left	0	0	0		0
		WB Thru	2	3200	1044	.326*	753
.235		WB Right	0	0	0		0

.409	Sum of Critical V/C Ratios (*)	.419
.100	Adjustment for Lost Time	.100
.509	INTERSECTION CAPACITY UTILIZATION	.519
A	Level of Service	A

	LOS	Maximum
ICU		-----
.60	A	
.70	B	
.80	C	
.90	D	
1.00	E	
n/a	F	

INTERSECTION CAPACITY UTILIZATION

Intersection: SR-170 SB ramp & Arleta

AM Count Date: 4/25/01

PM Count Date: 4/26/01

AM Peak Hour: 7:00A-8:00A

PM Peak Hour: 4:45P-5:45P

Analyst: TDS

Agency: Los Angeles

Hour	V/C	Movement	No. of Lanes	Capacity	AM Peak Hour Volume	AM Peak Hour V/C	PM Peak Hour Volume
		NB Left	0	0	0		0
		NB Thru	2	3200	605	.263*	1627
	.560*	NB Right	0	0	236		164
		SB Left	1	1600	384	.240*	142
	.089*	SB Thru	2	3200	1023	.320	542
	.169	SB Right	0	0	0		0
		EB Left	0	0	0		0
		EB Thru	0	0	0		0
		EB Right	0	0	0		0
		WB Left	0	0	0		0
		WB Thru	0	0	0		0
		WB Right	0	0	0		0

.649	Sum of Critical V/C Ratios (*)	.503
.100	Adjustment for Lost Time	.100
.749	INTERSECTION CAPACITY UTILIZATION	.603
C	Level of Service	B

ICU	LOS	Maximum
.60	A	
.70	B	
.80	C	
.90	D	
1.00	E	
n/a	F	

INTERSECTION CAPACITY UTILIZATION

Intersection: SR-170 SB ramp & Arleta

AM Count Date: Exist+Prj

PM Count Date: Exist+Prj

AM Peak Hour: 7:00A-8:00A

PM Peak Hour: 4:45P-5:45P

Analyst: TDS

Agency: Los Angeles

Hour	V/C	Movement	No. of Lanes	Capacity	AM Peak Hour Volume	AM Peak Hour V/C	PM Peak Hour Volume
		NB Left	0	0	0		0
		NB Thru	2	3200	605	.263*	1627
.601*		NB Right	0	0	236		296
		SB Left	1	1600	384	.240*	142
.089*		SB Thru	2	3200	1023	.320	542
.169		SB Right	0	0	0		0
		EB Left	0	0	0		0
		EB Thru	0	0	0		0
		EB Right	0	0	0		0
		WB Left	0	0	0		0
		WB Thru	0	0	0		0
		WB Right	0	0	0		0

.690	Sum of Critical V/C Ratios (*)	.503
.100	Adjustment for Lost Time	.100
.790	INTERSECTION CAPACITY UTILIZATION	.603
C	Level of Service	B

ICU	LOS	Maximum
-----		-----
.60	A	
.70	B	
.80	C	
.90	D	
1.00	E	
n/a	F	

INTERSECTION CAPACITY UTILIZATION

Intersection: SR-170 SB ramp & Arleta

AM Count Date: Exist+Prj w/Mit

PM Count Date: Exist+Prj w/Mit

AM Peak Hour: 7:00A-8:00A

PM Peak Hour: 4:45P-5:45P

Analyst: TDS

Agency: Los Angeles

Hour	V/C	Movement	No. of Lanes	Capacity	AM Peak Hour Volume	AM Peak Hour V/C	PM Peak Hour Volume
		NB Left	0	0	0		0
		NB Thru	2	3200	605	.263*	1627
	.592*	NB Right	0	0	236		266
		SB Left	1	1600	384	.240*	142
	.089*	SB Thru	2	3200	1023	.320	542
	.169	SB Right	0	0	0		0
		EB Left	0	0	0		0
		EB Thru	0	0	0		0
		EB Right	0	0	0		0
		WB Left	0	0	0		0
		WB Thru	0	0	0		0
		WB Right	0	0	0		0

.681	Sum of Critical V/C Ratios (*)	.503
.100	Adjustment for Lost Time	.100
.781	INTERSECTION CAPACITY UTILIZATION	.603
C	Level of Service	B

	LOS	Maximum
ICU		-----
.60	A	
.70	B	
.80	C	
.90	D	
1.00	E	
n/a	F	

INTERSECTION CAPACITY UTILIZATION

Intersection: Sunland Blvd & I-5 NB on/off

AM Count Date: 4/24/01

PM Count Date: 4/25/01

AM Peak Hour: 7:15A-8:15A

PM Peak Hour: 4:30P-5:30P

Analyst: TDS

Agency: Los Angeles

Hour	V/C	Movement	No. of Lanes	Capacity	AM Peak Hour Volume	V/C	PM Peak Volume
.171*	.233	NB Left	1	1600	287	.179*	274
		NB Thru	2	3200	469	.147	745
		NB Right	0	0	0		0
		SB Left	0	0	0		0
.171*		SB Thru	2	3200	952	.373*	441
		SB Right	0	0	240		107
		EB Left	0	0	0		0
		EB Thru	0	0	0		0
.016		EB Right	1	1600	177	.111	26
		WB Left	0.5		177		237
.152*		WB Thru	0.5	1600	0	.111*	6
.185		WB Right	1	1600	151	.094	296

.494	Sum of Critical V/C Ratios (*)	.663
.100	Adjustment for Lost Time	.100
.594	INTERSECTION CAPACITY UTILIZATION	.763
A	Level of Service	C

	LOS	Maximum
ICU		-----
.60	A	
.70	B	
.80	C	
.90	D	
1.00	E	
n/a	F	

INTERSECTION CAPACITY UTILIZATION

Intersection: Sunland Blvd & I-5 NB on/off

AM Count Date: Exist+Prj

PM Count Date: Exist+Prj

AM Peak Hour: 7:15A-8:15A

PM Peak Hour: 4:30P-5:30P

Analyst: TDS

Agency: Los Angeles

Hour	V/C	Movement	No. of Lanes	Capacity	AM Peak Hour Volume	V/C	PM Peak Volume
.171*	.233	NB Left	1	1600	287	.179*	274
		NB Thru	2	3200	469	.147	745
		NB Right	0	0	0		0
		SB Left	0	0	0		0
.171*		SB Thru	2	3200	952	.373*	441
		SB Right	0	0	240		107
		EB Left	0	0	0		0
		EB Thru	0	0	0		0
.016		EB Right	1	1600	177	.111	26
		WB Left	0.5		177		237
.152*		WB Thru	0.5	1600	0	.111*	6
.185		WB Right	1	1600	151	.094	296

.494	Sum of Critical V/C Ratios (*)	.663
.100	Adjustment for Lost Time	.100
.594	INTERSECTION CAPACITY UTILIZATION	.763
A	Level of Service	C

	LOS	Maximum
ICU		-----
.60	A	
.70	B	
.80	C	
.90	D	
1.00	E	
n/a	F	

INTERSECTION CAPACITY UTILIZATION

Intersection: Sunland Blvd & I-5 NB on/off

AM Count Date: Exist+Prj w/Mit

PM Count Date: Exist+Prj w/Mit

AM Peak Hour: 7:15A-8:15A

PM Peak Hour: 4:30P-5:30P

Analyst: TDS

Agency: Los Angeles

Hour	V/C	Movement	No. of Lanes	Capacity	AM Peak Hour Volume	V/C	PM Peak Volume
.171*	.233	NB Left	1	1600	287	.179*	274
		NB Thru	2	3200	469	.147	745
		NB Right	0	0	0		0
		SB Left	0	0	0		0
.171*		SB Thru	2	3200	952	.373*	441
		SB Right	0	0	240		107
		EB Left	0	0	0		0
		EB Thru	0	0	0		0
.016		EB Right	1	1600	177	.111	26
		WB Left	0.5		177		237
.152*		WB Thru	0.5	1600	0	.111*	6
.185		WB Right	1	1600	151	.094	296

.494	Sum of Critical V/C Ratios (*)	.663
.100	Adjustment for Lost Time	.100
.594	INTERSECTION CAPACITY UTILIZATION	.763
A	Level of Service	C

	LOS	Maximum
ICU		-----
.60	A	
.70	B	
.80	C	
.90	D	
1.00	E	
n/a	F	

INTERSECTION CAPACITY UTILIZATION

Intersection: Sunland Blvd & I-5 SB on/off

AM Count Date: 4/26/01

PM Count Date: 4/24/01

AM Peak Hour: 7:00A-8:00A

PM Peak Hour: 4:15P-5:15P

Analyst: TDS

Agency: Los Angeles

Hour	V/C	Movement	No. of Lanes	Capacity	AM Peak Hour Volume	AM Peak Hour V/C	PM Peak Volume
		NB Left	0	0	0		0
		NB Thru	2	3200	710	.294*	1041
.375*		NB Right	0	0	232		159
		SB Left	1	1600	434	.271*	230
.144*		SB Thru	2	3200	752	.235	733
.229		SB Right	0	0	0		0
		EB Left	1	1600	127	.079	226
.141		EB Thru	0.5	1600	112	.104*	139
.229*		EB Right	0.5		54		228
		WB Left	0	0	0		0
		WB Thru	0	0	0		0
		WB Right	0	0	0		0

.748	Sum of Critical V/C Ratios (*)	.669
.100	Adjustment for Lost Time	.100
.848	INTERSECTION CAPACITY UTILIZATION	.769
D	Level of Service	C

	LOS	Maximum
ICU		-----
.60	A	
.70	B	
.80	C	
.90	D	
1.00	E	
n/a	F	

INTERSECTION CAPACITY UTILIZATION

Intersection: Sunland Blvd & I-5 SB on/off

AM Count Date: Exist+Prj

PM Count Date: Exist+Prj

AM Peak Hour: 7:00A-8:00A

PM Peak Hour: 4:15P-5:15P

Analyst: TDS

Agency: Los Angeles

Hour	V/C	Movement	No. of Lanes	Capacity	AM Peak Hour Volume	AM Peak Hour V/C	PM Peak Hour Volume
		NB Left	0	0	0		0
		NB Thru	2	3200	710	.294*	1041
	.375*	NB Right	0	0	232		159
		SB Left	1	1600	434	.271*	230
	.144*	SB Thru	2	3200	752	.235	733
	.229	SB Right	0	0	0		0
		EB Left	1	1600	127	.079	226
	.141	EB Thru	0.5	1600	112	.104*	139
	.229*	EB Right	0.5		54		228
		WB Left	0	0	0		0
		WB Thru	0	0	0		0
		WB Right	0	0	0		0

.748	Sum of Critical V/C Ratios (*)	.669
.100	Adjustment for Lost Time	.100
.848	INTERSECTION CAPACITY UTILIZATION	.769
D	Level of Service	C

	LOS	Maximum
ICU		-----
.60	A	
.70	B	
.80	C	
.90	D	
1.00	E	
n/a	F	

INTERSECTION CAPACITY UTILIZATION

Intersection: Sunland Blvd & I-5 SB on/off

AM Count Date: Exist+Prj w/Mit

PM Count Date: Exist+Prj w/Mit

AM Peak Hour: 7:00A-8:00A

PM Peak Hour: 4:15P-5:15P

Analyst: TDS

Agency: Los Angeles

Hour	V/C	Movement	No. of Lanes	Capacity	AM Peak Hour Volume	AM Peak Hour V/C	PM Peak Hour Volume
		NB Left	0	0	0		0
		NB Thru	2	3200	710	.294*	1041
	.375*	NB Right	0	0	232		159
		SB Left	1	1600	434	.271*	230
	.144*	SB Thru	2	3200	752	.235	733
	.229	SB Right	0	0	0		0
		EB Left	1	1600	127	.079	226
	.141	EB Thru	0.5	1600	112	.104*	139
	.229*	EB Right	0.5		54		228
		WB Left	0	0	0		0
		WB Thru	0	0	0		0
		WB Right	0	0	0		0

.748	Sum of Critical V/C Ratios (*)	.669
.100	Adjustment for Lost Time	.100
.848	INTERSECTION CAPACITY UTILIZATION	.769
D	Level of Service	C

ICU	LOS	Maximum
.60	A	
.70	B	
.80	C	
.90	D	
1.00	E	
n/a	F	

INTERSECTION CAPACITY UTILIZATION

Intersection: I-5 NB on/Rincon & Sheldon St

AM Count Date: 4/30/01

PM Count Date: 4/24/01

AM Peak Hour: 7:00A-8:00A

PM Peak Hour: 4:30P-5:30P

Analyst: TDS

Agency: Los Angeles

Hour	V/C	Movement	No. of Lanes	Capacity	AM Peak Hour Volume	V/C	PM Peak Volume
		NB Left	0	0	0		0
		NB Thru	0	0	0		0
		NB Right	0	0	0		0
		SB Left	0	0	2		0
.008*		SB Thru	1	1600	2	.016*	0
		SB Right	0	0	22		13
		EB Left	0	0	6		29
.388*		EB Thru	2	3200	1108	.401*	1001
		EB Right	0	0	168		213
		WB Left	1	1600	192	.120*	127
.079*		WB Thru	2	3200	1078	.338	682
.215		WB Right	0	0	4		6

.475	Sum of Critical V/C Ratios (*)	.537
.100	Adjustment for Lost Time	.100
.575	INTERSECTION CAPACITY UTILIZATION	.637
A	Level of Service	B

ICU	LOS	Maximum
-----	-----	-----
.60	A	
.70	B	
.80	C	
.90	D	
1.00	E	
n/a	F	

INTERSECTION CAPACITY UTILIZATION

Intersection: I-5 NB on/Rincon & Sheldon St

AM Count Date: Exist+Prj

PM Count Date: Exist+Prj

AM Peak Hour: 7:00A-8:00A

PM Peak Hour: 4:30P-5:30P

Analyst: TDS

Agency: Los Angeles

Hour	V/C	Movement	No. of Lanes	Capacity	AM Peak Hour Volume	V/C	PM Peak Volume
		NB Left	0	0	0		0
		NB Thru	0	0	0		0
		NB Right	0	0	0		0
		SB Left	0	0	2		0
.008*		SB Thru	1	1600	2	.016*	0
		SB Right	0	0	22		13
		EB Left	0	0	6		29
.388*		EB Thru	2	3200	1108	.401*	1001
		EB Right	0	0	168		213
		WB Left	1	1600	192	.120*	127
.079*		WB Thru	2	3200	1078	.338	682
.215		WB Right	0	0	4		6

.475	Sum of Critical V/C Ratios (*)	.537
.100	Adjustment for Lost Time	.100
.575	INTERSECTION CAPACITY UTILIZATION	.637
A	Level of Service	B

	LOS	Maximum
ICU		-----
.60	A	
.70	B	
.80	C	
.90	D	
1.00	E	
n/a	F	

INTERSECTION CAPACITY UTILIZATION

Intersection: I-5 NB on/Rincon & Sheldon St

AM Count Date: Exist+Prj w/Mit

PM Count Date: Exist+Prj w/Mit

AM Peak Hour: 7:00A-8:00A

PM Peak Hour: 4:30P-5:30P

Analyst: TDS

Agency: Los Angeles

Hour	V/C	Movement	No. of Lanes	Capacity	AM Peak Hour Volume	V/C	PM Peak Volume
		NB Left	0	0	0		0
		NB Thru	0	0	0		0
		NB Right	0	0	0		0
		SB Left	0	0	2		0
.008*		SB Thru	1	1600	2	.016*	0
		SB Right	0	0	22		13
		EB Left	0	0	6		29
.388*		EB Thru	2	3200	1108	.401*	1001
		EB Right	0	0	168		213
		WB Left	1	1600	192	.120*	127
.079*		WB Thru	2	3200	1078	.338	682
.215		WB Right	0	0	4		6

.475	Sum of Critical V/C Ratios (*)	.537
.100	Adjustment for Lost Time	.100
.575	INTERSECTION CAPACITY UTILIZATION	.637
A	Level of Service	B

ICU	LOS	Maximum
-----		-----
.60	A	
.70	B	
.80	C	
.90	D	
1.00	E	
n/a	F	

INTERSECTION CAPACITY UTILIZATION

Intersection: Laurel Cyn & I-5 SB on/off

AM Count Date: 4/26/01

PM Count Date: 4/24/01

AM Peak Hour: 7:00A-8:00A

PM Peak Hour: 4:45P-5:45P

Analyst: TDS

Agency: Los Angeles

Hour	V/C	Movement	No. of Lanes	Capacity	AM Peak Hour Volume	V/C	PM Peak Volume
		NB Left	0	0	0		0
		NB Thru	2	3200	704	.235*	1087
		NB Right	0	0	48		77
		SB Left	1	1600	225	.141*	182
		SB Thru	2	3200	659	.206	658
		SB Right	0	0	0		0
		EB Left	0	0	0		0
		EB Thru	0	0	0		0
		EB Right	0	0	0		0
		WB Left	1	1600	70	.044*	124
		WB Thru	0	0	0		0
		WB Right	1	1600	173	.108	185

.556	Sum of Critical V/C Ratios (*)	.420
.100	Adjustment for Lost Time	.100
.656	INTERSECTION CAPACITY UTILIZATION	.520
B	Level of Service	A

	LOS	Maximum
ICU		-----
.60	A	
.70	B	
.80	C	
.90	D	
1.00	E	
n/a	F	

INTERSECTION CAPACITY UTILIZATION

Intersection: Laurel Cyn & I-5 SB on/off

AM Count Date: Exist+Prj

PM Count Date: Exist+Prj

AM Peak Hour: 7:00A-8:00A

PM Peak Hour: 4:45P-5:45P

Analyst: TDS

Agency: Los Angeles

Hour	V/C	Movement	No. of Lanes	Capacity	AM Peak Hour Volume	V/C	PM Peak Volume
		NB Left	0	0	0		0
		NB Thru	2	3200	704	.235*	1087
.364*		NB Right	0	0	48		77
		SB Left	1	1600	225	.141*	230
.144*		SB Thru	2	3200	659	.206	658
.206		SB Right	0	0	0		0
		EB Left	0	0	0		0
		EB Thru	0	0	0		0
		EB Right	0	0	0		0
		WB Left	1	1600	70	.044*	124
.078*		WB Thru	0	0	0		0
		WB Right	1	1600	173	.108	185
.116							

.586	Sum of Critical V/C Ratios (*)	.420
.100	Adjustment for Lost Time	.100
.686	INTERSECTION CAPACITY UTILIZATION	.520
B	Level of Service	A

ICU	LOS	Maximum
.60	A	
.70	B	
.80	C	
.90	D	
1.00	E	
n/a	F	

INTERSECTION CAPACITY UTILIZATION

Intersection: Laurel Cyn & I-5 SB on/off

AM Count Date: Exist+Prj w/Mit

PM Count Date: Exist+Prj w/Mit

AM Peak Hour: 7:00A-8:00A

PM Peak Hour: 4:45P-5:45P

Analyst: TDS

Agency: Los Angeles

Hour	V/C	Movement	No. of Lanes	Capacity	AM Peak Hour Volume	V/C	PM Peak Volume
		NB Left	0	0	0		0
		NB Thru	2	3200	704	.235*	1087
	.364*	NB Right	0	0	48		77
		SB Left	1	1600	225	.141*	219
	.137*	SB Thru	2	3200	659	.206	658
	.206	SB Right	0	0	0		0
		EB Left	0	0	0		0
		EB Thru	0	0	0		0
		EB Right	0	0	0		0
		WB Left	1	1600	70	.044*	124
	.078*	WB Thru	0	0	0		0
		WB Right	1	1600	173	.108	185
	.116						

.579	Sum of Critical V/C Ratios (*)	.420
.100	Adjustment for Lost Time	.100
.679	INTERSECTION CAPACITY UTILIZATION	.520
B	Level of Service	A

ICU	LOS	Maximum
-----		-----
.60	A	
.70	B	
.80	C	
.90	D	
1.00	E	
n/a	F	

INTERSECTION CAPACITY UTILIZATION

Intersection: Laurel Cyn & I-5 NB off

AM Count Date: 4/26/01

PM Count Date: 4/25/01

AM Peak Hour: 7:30A-8:30A

PM Peak Hour: 4:00P-5:00P

Analyst: TDS

Agency: Los Angeles

Hour	V/C	Movement	No. of Lanes	Capacity	AM Peak Hour Volume	V/C	PM Peak Volume
		NB Left	0	0	0		0
		NB Thru	2	3200	348	.114	922
.300*		NB Right	0	0	18		38
		SB Left	1	1600	15	.009	49
.031*		SB Thru	2	3200	675	.211*	643
.201		SB Right	0	0	0		0
		EB Left	1	1600	12	.008*	56
.035*		EB Thru	0	0	6		6
		EB Right	1	1600	202	.126	169
.106		WB Left	0	0	19		17
		WB Thru	1	1600	0	.023*	0
.034*		WB Right	0	0	17		38
		Right Turn Adjustment			EBR	.107*	

		LOS	Maximum
-----+			
	Sum of Critical V/C Ratios (*)	.349	
.400			
	Adjustment for Lost Time	.100	
.100			
	INTERSECTION CAPACITY UTILIZATION	.449	
.500			
	Level of Service	A	
A			
ICU			

		A	
.60		B	
.70		C	
.80		D	
.90		E	
1.00		F	
n/a			

INTERSECTION CAPACITY UTILIZATION

Intersection: Laurel Cyn & I-5 NB off

AM Count Date: Exist+Prj

PM Count Date: Exist+Prj

AM Peak Hour: 7:30A-8:30A

PM Peak Hour: 4:00P-5:00P

Analyst: TDS

Agency: Los Angeles

Hour	V/C	Movement	No. of Lanes	Capacity	AM Peak Hour Volume	V/C	PM Peak Volume
		NB Left	0	0	0		0
		NB Thru	2	3200	348	.114	922
.300*		NB Right	0	0	18		38
		SB Left	1	1600	15	.009	49
.031*		SB Thru	2	3200	675	.211*	643
.201		SB Right	0	0	0		0
		EB Left	1	1600	12	.008*	56
.035*		EB Thru	0	0	6		6
		EB Right	1	1600	202	.126	169
.106		WB Left	0	0	19		17
		WB Thru	1	1600	0	.023*	0
.034*		WB Right	0	0	17		38
		Right Turn Adjustment			EBR	.107*	

		LOS	Maximum
-----+	Sum of Critical V/C Ratios (*)	.349	
.400	Adjustment for Lost Time	.100	
.100	INTERSECTION CAPACITY UTILIZATION	.449	
.500	Level of Service	A	
A			
ICU			

.60		A	
.70		B	
.80		C	
.90		D	
1.00		E	
n/a		F	

INTERSECTION CAPACITY UTILIZATION

Intersection: Laurel Cyn & I-5 NB off

AM Count Date: Exist+Prj w/Mit

PM Count Date: Exist+Prj w/Mit

AM Peak Hour: 7:30A-8:30A

PM Peak Hour: 4:00P-5:00P

Analyst: TDS

Agency: Los Angeles

Hour	V/C	Movement	No. of Lanes	Capacity	AM Peak Hour Volume	V/C	PM Peak Volume
		NB Left	0	0	0		0
		NB Thru	2	3200	348	.114	922
.300*		NB Right	0	0	18		38
		SB Left	1	1600	15	.009	49
.031*		SB Thru	2	3200	675	.211*	643
.201		SB Right	0	0	0		0
		EB Left	1	1600	12	.008*	56
.035*		EB Thru	0	0	6		6
		EB Right	1	1600	202	.126	169
.106		WB Left	0	0	19		17
		WB Thru	1	1600	0	.023*	0
.034*		WB Right	0	0	17		38
		Right Turn Adjustment			EBR	.107*	

		LOS	Maximum
	Sum of Critical V/C Ratios (*)	.349	
	Adjustment for Lost Time	.100	
	INTERSECTION CAPACITY UTILIZATION	.449	
A	Level of Service	A	
ICU			
		A	
		B	
		C	
		D	
		E	
		F	
n/a			

INTERSECTION CAPACITY UTILIZATION

Intersection: I-5 NB on/off & Osborne St

AM Count Date: 4/26/01

PM Count Date: 4/25/01

AM Peak Hour: 7:15A-8:15A

PM Peak Hour: 5:00P-6:00P

Analyst: TDS

Agency: Los Angeles

Hour	V/C	Movement	No. of Lanes	Capacity	AM Peak Hour Volume	V/C	PM Peak Volume
{.187}* .187		NB Left	1.5		269	.168*	394
		NB Thru	0	3200	0		0
		NB Right	0.5		332	.208	205
.046		SB Left	0	0	0		0
		SB Thru	0	0	0		0
		SB Right	1	1600	37	.023	74
.417* .265		EB Left	0	0	14	{.009}*	0
		EB Thru	2	3200	839	.267	1335
		EB Right	1	1600	677	.423	424
.323		WB Left	0	0	0		0
		WB Thru	3	4800	1004	.314*	1086
		WB Right	0	0	631	.394	466
		Right Turn Adjustment			SBR	.016*	

		+-----+	
-----+			
.604	Sum of Critical V/C Ratios (*)	.507	
.100	Adjustment for Lost Time	.100	
INTERSECTION CAPACITY UTILIZATION	.607	.704	
C	Level of Service	B	
ICU		LOS	Maximum
-----			-----
.60		A	
.70		B	
.80		C	
.90		D	
1.00		E	
n/a		F	

INTERSECTION CAPACITY UTILIZATION

Intersection: I-5 NB on/off & Osborne St

AM Count Date: Exist+Prj

PM Count Date: Exist+Prj

AM Peak Hour: 7:15A-8:15A

PM Peak Hour: 5:00P-6:00P

Analyst: TDS

Agency: Los Angeles

Hour	V/C	Movement	No. of Lanes	Capacity	AM Peak Hour Volume	V/C	PM Peak Volume
{.187}* .187		NB Left	1.5		269	.168*	394
		NB Thru	0	3200	0		0
		NB Right	0.5		332	.208	205
.046		SB Left	0	0	0		0
		SB Thru	0	0	0		0
		SB Right	1	1600	37	.023	74
.417* .265		EB Left	0	0	14	{.009}*	0
		EB Thru	2	3200	839	.267	1335
		EB Right	1	1600	677	.423	424
.339 .385		WB Left	0	0	0		0
		WB Thru	3	4800	1004	.314*	1086
		WB Right	0	0	631	.394	616
		Right Turn Adjustment			SBR	.016*	

INTERSECTION CAPACITY UTILIZATION

Intersection: I-5 NB on/off & Osborne St

AM Count Date: Exist+Prj w/Mit

PM Count Date: Exist+Prj w/Mit

AM Peak Hour: 7:15A-8:15A

PM Peak Hour: 5:00P-6:00P

Analyst: TDS

Agency: Los Angeles

Hour	V/C	Movement	No. of Lanes	Capacity	AM Peak Hour Volume	V/C	PM Peak Volume
{.187}* .187		NB Left	1.5		269	.168*	394
		NB Thru	0	3200	0		0
		NB Right	0.5		332	.208	205
.046		SB Left	0	0	0		0
		SB Thru	0	0	0		0
		SB Right	1	1600	37	.023	74
.417* .265		EB Left	0	0	14	{.009}*	0
		EB Thru	2	3200	839	.267	1335
		EB Right	1	1600	677	.423	424
.339 .364		WB Left	0	0	0		0
		WB Thru	3	4800	1004	.314*	1086
		WB Right	0	0	631	.394	582
		Right Turn Adjustment			SBR	.016*	

INTERSECTION CAPACITY UTILIZATION

Intersection: I-5 SB on/off & Osborne St

AM Count Date: 4/26/01

PM Count Date: 4/25/01

AM Peak Hour: 7:15A-8:15A

PM Peak Hour: 5:00P-6:00P

Analyst: TDS

Agency: Los Angeles

Hour	V/C	Movement	No. of Lanes	Capacity	AM Peak Hour Volume	V/C	PM Peak Volume
		NB Left	0	0	0		0
		NB Thru	0	0	0		0
		NB Right	1	1600	0	.000	0
		SB Left	1	1600	303	.189	470
		SB Thru	0.5	1600	15	.209*	6
		SB Right	0.5		320		571
		EB Left	0	0	0		0
		EB Thru	2	3200	985	.498*	1050
		EB Right	0	0	609		428
		WB Left	1	1600	65	.041*	15
		WB Thru	2	3200	1127	.352	1361
		WB Right	1	1600	537	.336	344

.832	Sum of Critical V/C Ratios (*)	.748
.100	Adjustment for Lost Time	.100
.932	INTERSECTION CAPACITY UTILIZATION	.848
E	Level of Service	D

	LOS	Maximum
ICU		-----
.60	A	
.70	B	
.80	C	
.90	D	
1.00	E	
n/a	F	

INTERSECTION CAPACITY UTILIZATION

Intersection: I-5 SB on/off & Osborne St

AM Count Date: Exist+Prj

PM Count Date: Exist+Prj

AM Peak Hour: 7:15A-8:15A

PM Peak Hour: 5:00P-6:00P

Analyst: TDS

Agency: Los Angeles

Hour	V/C	Movement	No. of Lanes	Capacity	AM Peak Hour Volume	V/C	PM Peak Volume
		NB Left	0	0	0		0
		NB Thru	0	0	0		0
		NB Right	1	1600	0	.000	0
		SB Left	1	1600	303	.189	470
		SB Thru	0.5	1600	15	.209*	6
		SB Right	0.5		320		571
		EB Left	0	0	0		0
		EB Thru	2	3200	985	.498*	1050
		EB Right	0	0	609		428
		WB Left	1	1600	65	.041*	15
		WB Thru	2	3200	1127	.352	1361
		WB Right	1	1600	537	.336	344

.832	Sum of Critical V/C Ratios (*)	.748
.100	Adjustment for Lost Time	.100
.932	INTERSECTION CAPACITY UTILIZATION	.848
E	Level of Service	D

	LOS	Maximum
ICU		-----
.60	A	
.70	B	
.80	C	
.90	D	
1.00	E	
n/a	F	

INTERSECTION CAPACITY UTILIZATION

Intersection: I-5 SB on/off & Osborne St

AM Count Date: Exist+Prj w/Mit

PM Count Date: Exist+Prj w/Mit

AM Peak Hour: 7:15A-8:15A

PM Peak Hour: 5:00P-6:00P

Analyst: TDS

Agency: Los Angeles

Hour	V/C	Movement	No. of Lanes	Capacity	AM Peak Hour Volume	V/C	PM Peak Volume
		NB Left	0	0	0		0
		NB Thru	0	0	0		0
		NB Right	1	1600	0	.000	0
		SB Left	1	1600	303	.189	470
		SB Thru	0.5	1600	15	.209*	6
		SB Right	0.5		320		571
		EB Left	0	0	0		0
		EB Thru	2	3200	985	.498*	1050
		EB Right	0	0	609		428
		WB Left	1	1600	65	.041*	15
		WB Thru	2	3200	1127	.352	1361
		WB Right	1	1600	537	.336	344

.832	Sum of Critical V/C Ratios (*)	.748
.100	Adjustment for Lost Time	.100
.932	INTERSECTION CAPACITY UTILIZATION	.848
E	Level of Service	D

	LOS	Maximum
ICU		-----
.60	A	
.70	B	
.80	C	
.90	D	
1.00	E	
n/a	F	

INTERSECTION CAPACITY UTILIZATION

Intersection: Laurel Cyn & Sheldon St

AM Count Date: 4/24/01

PM Count Date: 4/26/01

AM Peak Hour: 7:00A-8:00A

PM Peak Hour: 4:30P-5:30P

Analyst: TDS

Agency: Los Angeles

Hour	V/C	Movement	No. of Lanes	Capacity	AM Peak Hour Volume	V/C	PM Peak Volume
.023	.308*	NB Left	1	1600	43	.027*	36
		NB Thru	2	3200	315	.197	581
		NB Right	0	0	421	.263	405
.070*	.139	SB Left	1	1600	145	.091	112
		SB Thru	2	3200	658	.278*	362
		SB Right	0	0	231		84
.094*	.193	EB Left	1	1600	78	.049*	150
		EB Thru	2	3200	715	.253	501
		EB Right	0	0	95		116
.094	.193*	WB Left	1	1600	121	.076	150
		WB Thru	2	3200	806	.286*	501
		WB Right	0	0	110		116

.665	Sum of Critical V/C Ratios (*)	.640
.100	Adjustment for Lost Time	.100
INTERSECTION CAPACITY UTILIZATION	.740	.765
C	Level of Service	C

ICU	LOS	Maximum
-----	-----	-----
.60	A	
.70	B	
.80	C	
.90	D	
1.00	E	
n/a	F	

INTERSECTION CAPACITY UTILIZATION

Intersection: Laurel Cyn & Sheldon St

AM Count Date: Exist+Prj

PM Count Date: Exist+Prj

AM Peak Hour: 7:00A-8:00A

PM Peak Hour: 4:30P-5:30P

Analyst: TDS

Agency: Los Angeles

Hour	V/C	Movement	No. of Lanes	Capacity	AM Peak Hour Volume	V/C	PM Peak Volume
.023	.308*	NB Left	1	1600	43	.027*	36
		NB Thru	2	3200	315	.197	581
		NB Right	0	0	421	.263	405
.070*	.139	SB Left	1	1600	145	.091	112
		SB Thru	2	3200	658	.278*	362
		SB Right	0	0	231		84
.094*	.193	EB Left	1	1600	78	.049*	150
		EB Thru	2	3200	715	.253	501
		EB Right	0	0	95		116
.124	.240*	WB Left	1	1600	121	.076	198
		WB Thru	2	3200	806	.286*	651
		WB Right	0	0	110		116

.712	Sum of Critical V/C Ratios (*)	.640
.100	Adjustment for Lost Time	.100
.812	INTERSECTION CAPACITY UTILIZATION	.740
D	Level of Service	C

ICU	LOS	Maximum
.60	A	
.70	B	
.80	C	
.90	D	
1.00	E	
n/a	F	

INTERSECTION CAPACITY UTILIZATION

Intersection: Laurel Cyn & Sheldon St

AM Count Date: Exist+Prj w/Mit

PM Count Date: Exist+Prj w/Mit

AM Peak Hour: 7:00A-8:00A

PM Peak Hour: 4:30P-5:30P

Analyst: TDS

Agency: Los Angeles

Hour	V/C	Movement	No. of Lanes	Capacity	AM Peak Hour Volume	V/C	PM Peak Volume
.023	.308*	NB Left	1	1600	43	.027*	36
		NB Thru	2	3200	315	.197	581
		NB Right	0	0	421	.263	405
.070*	.139	SB Left	1	1600	145	.091	112
		SB Thru	2	3200	658	.278*	362
		SB Right	0	0	231		84
.094*	.193	EB Left	1	1600	78	.049*	150
		EB Thru	2	3200	715	.253	501
		EB Right	0	0	95		116
.117	.229*	WB Left	1	1600	121	.076	187
		WB Thru	2	3200	806	.286*	617
		WB Right	0	0	110		116

.701	Sum of Critical V/C Ratios (*)	.640
.100	Adjustment for Lost Time	.100
.801	INTERSECTION CAPACITY UTILIZATION	.740
D	Level of Service	C

	LOS	Maximum
ICU		-----
.60	A	
.70	B	
.80	C	
.90	D	
1.00	E	
n/a	F	